

15 May 2025 File No.: 25-1781

Avant Group Inc. 5980 Tecumseh Road East Windsor, Ontario N8T 1E3

Via email: mohhanash81@gmail.com

Attention: Mohammad Hanash

Re: 619 Cabana Road West, Windsor, ON

**Traffic Impact Statement** 



A residential apartment building complex is proposed for lands on the south side of Cabana Road West, approximately 275m west of Dougall Avenue, in Windsor, Ontario. The site is currently occupied by a single-family residential dwelling. The area plan is illustrated on **Figure 1**. Cabana Road West is an east / west arterial roadway that begins at Huron Church Road and runs east to Dougall Avenue, where it transitions to Cabana Road East; it joins Division Road west of Walker Road and continues east through Essex County as County Road 42. The site is located north of the Roseland Golf and Curling Club.

The proposed site plan is illustrated on **Figure 2**; the development proposal consists of three eight-unit apartment buildings. One joint access is proposed at Cabana Road West; the joint access will service the three buildings. The developer is proposing nine parking spaces to accommodate each building, with one accessible space for each.

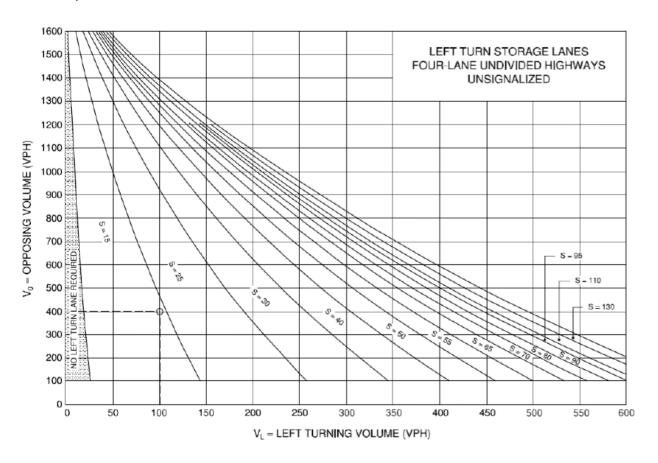
The purpose of this traffic impact brief is to address the proposed development's impact on Cabana Road West traffic operations; a traffic impact statement was deemed to be sufficient in addressing the subject development's traffic impact.

Trip generation for the proposed development was estimated from the Institute of Transportation Engineers (ITE) Trip Generation Manual (11<sup>th</sup> Edition). The dataset's average rate was used instead of the fitted curve equation because the value of the independent variables is in the lower range of the dataset; the fitted curve equation does not pass through the origin. The trip generation calculations are provided in **Appendix A**. ITE Land Use Code 220 – Multifamily Housing (Low-Rise) is the most appropriate code for the residential buildings. This land use code provides trip generation rates of 0.40 trips per dwelling unit in the AM peak hour, with 24% entering and 76% exiting, and 0.51 trips per dwelling unit in the PM peak hour, with 63% entering and 37% exiting. Accordingly, this portion of the development is expected to generate 10 trips in the AM peak hour, with 2 entering and 8 exiting, and 12 trips in the PM peak hour, with 8 entering and 4 exiting. Site generated traffic was distributed to and from Cabana Road West based on the east / west flow of traffic noted in recent turning movement counts collected on Cabana Road West; this data indicated a 50-50 east / west split. **Figure 3** depicts the anticipated traffic distribution.



The proposed northbound stop-controlled site access at Cabana Road West is to be comprised of a shared lane on the northbound approach (approximately 6m in width) and a four-lane cross-section on Cabana Road West. There are no dedicated turning lanes on any approach. Based on the anticipated site generated traffic volumes, it is anticipated that the proposed site access will exhibit a satisfactory level of service during the critical peak hours.

According to Exhibit 9A-31 of the MTO Design Supplement (October 2023) to the TAC Geometric Design Guide for Canadian Roads (June 2017), shown below, the left turning volumes are simply too low to warrant a dedicated westbound left turn lane at the subject site access; no geometric and / or traffic control improvements are warranted for this section of Cabana Road West:



A sight line analysis was completed for the site accesses at Cabana Road West. The analysis was completed per the TAC Geometric Design Guide for Canadian Roads (2017). The speed limit on Cabana Road West is 50 km/h, so the analysis was completed for a 60 km/h design speed; a passenger car was selected as the design vehicle. According to the TAC Geometric Design Guide for Canadian Roads (2017), the sight line should be evaluated with the design vehicle located at 4.4m from the edge of the nearest travelled lane. As calculated in **Appendix B**, the minimum intersection sight distance is 125m for the worst-case left turn egress maneuver and 108m for the less-critical right turn egress maneuver. The critical "left turn from stop" and the less-critical "right turn from stop" is illustrated on **Figure 4** for the site access. Based on the illustrated sight lines, it is the engineers' opinion that there is sufficient sight distance for safe egress from the proposed site access.



Therefore, based on the results of the analysis, it is the engineers' opinion that the proposed development will not adversely impact area traffic operations. Geometric and / or traffic control improvements are not required to support the proposed residential development.

All of which is respectfully submitted,

**RC Spencer Associates Inc.** 

Aaron D. Blata, M.Eng., P.Eng., PTOE, RSP1

Consulting Engineer, Road Safety

Professional Traffic Operations Engineer

**Associate / Leamington Office Manager** 



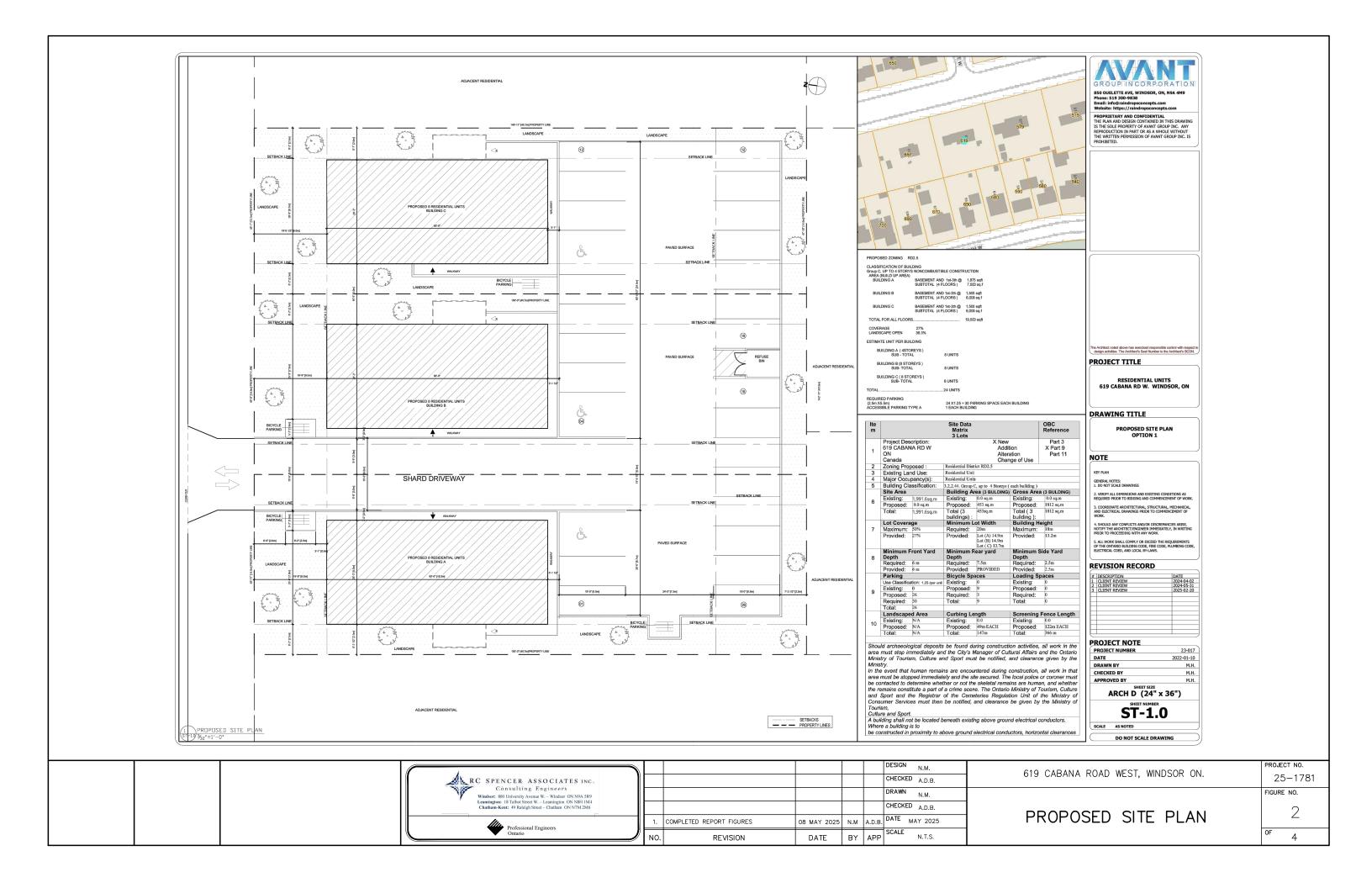
Richard C. Spercer, M.A.Sc., P.Eng., PE Professional & Consulting Engineer &

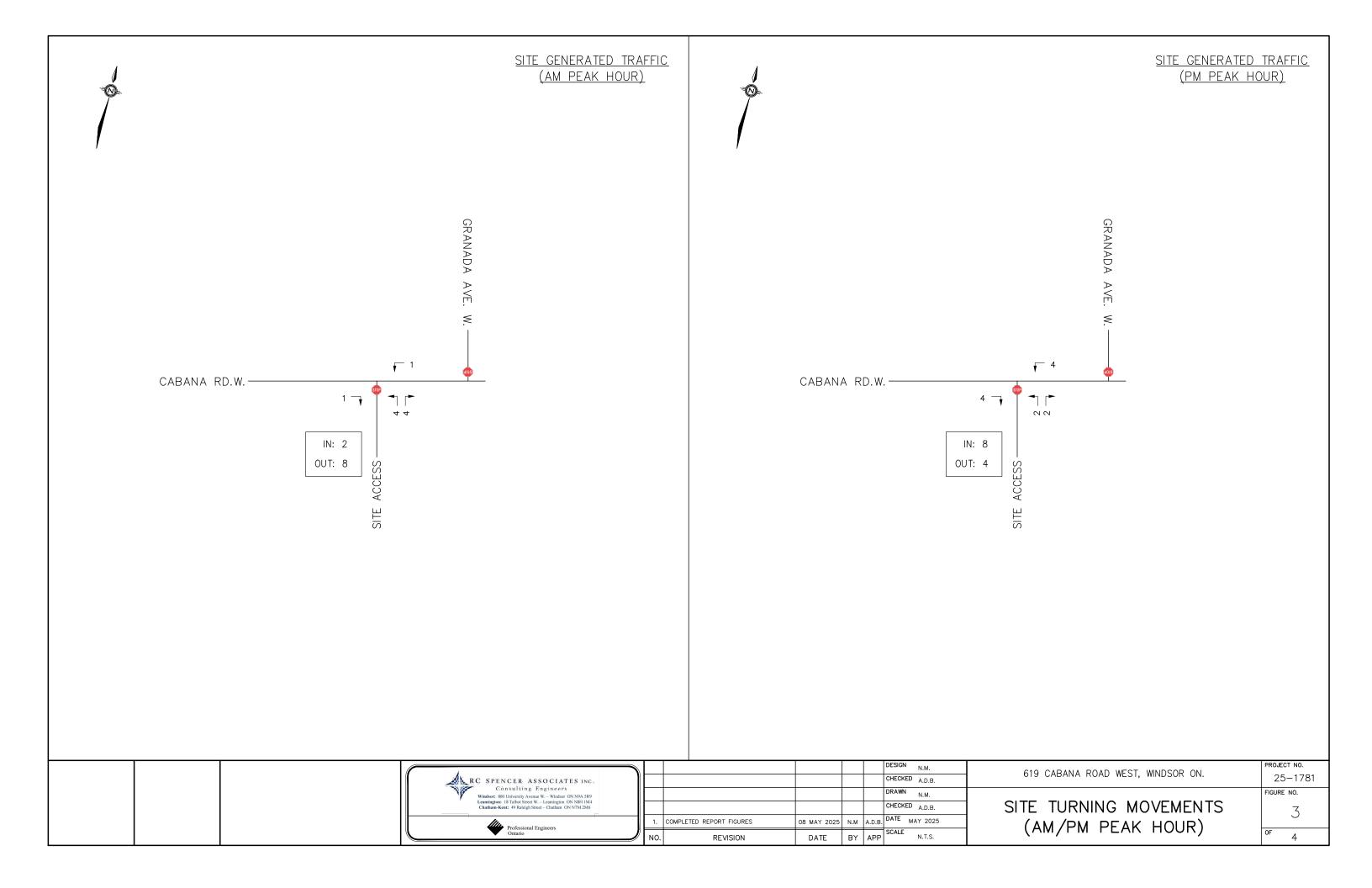
Fellow ITE Member

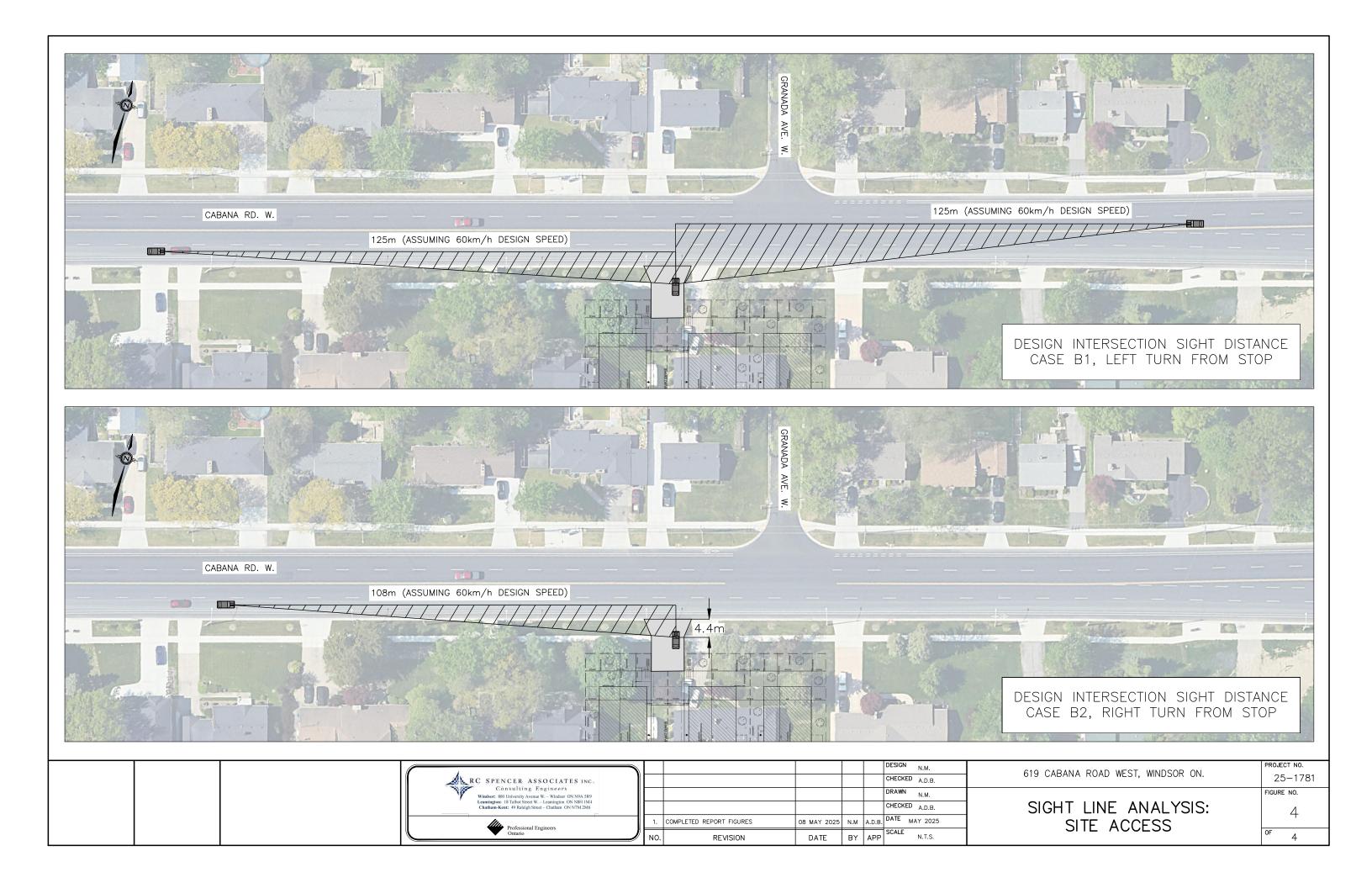
**President / Windsor Office Manager** 











## **Appendix A**

# ITE TRIP GENERATION MANUAL – 11<sup>TH</sup> EDITION REFERENCES

### Multifamily Housing (Low-Rise)

Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

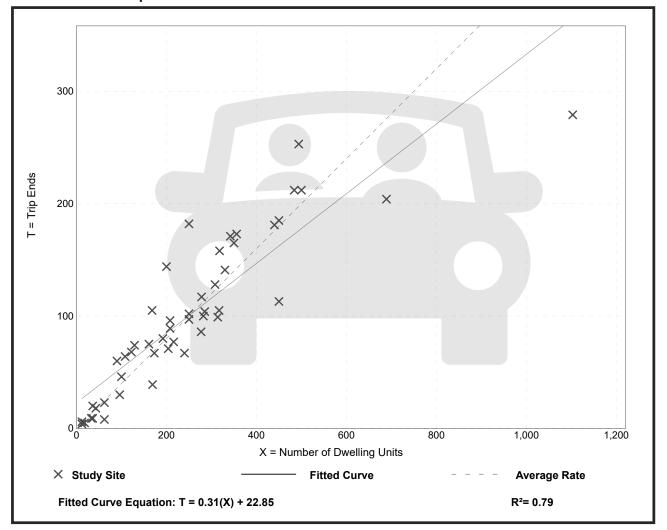
Number of Studies: 49 Avg. Num. of Dwelling Units: 249

Directional Distribution: 24% entering, 76% exiting

**Vehicle Trip Generation per Dwelling Unit** 

Average Rate	Range of Rates	Standard Deviation
0.40	0.13 - 0.73	0.12

#### **Data Plot and Equation**



## **Multifamily Housing (Low-Rise)**

Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

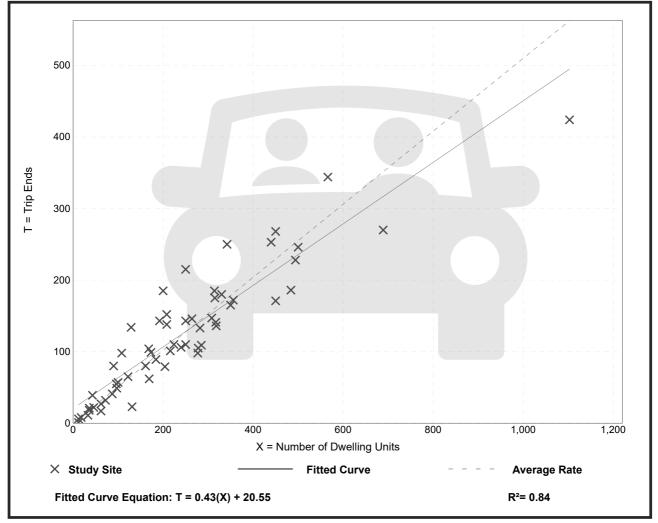
Number of Studies: 59 Avg. Num. of Dwelling Units: 241

Directional Distribution: 63% entering, 37% exiting

#### **Vehicle Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation
0.51	0.08 - 1.04	0.15

#### **Data Plot and Equation**



#### **Proposed Site Development Trip Generation and Distribution**

**Project:** 619 Cabana Road West

Site: Windsor, Ontario

Assumed Land Use (1): Multifamily Housing (Low-rise) - ITE No. 220

Average Vehicle Trip Ends vs.: Dwelling Units

ITE Trip Generation Data collected on a: Weekday

AM Peak Hour: 0.40 = Average Rate 24 % Entering 76 % Exiting

PM Peak Hour: 0.51 = Average Rate 63 % Entering 37 % Exiting

Assumed Land Use (1): Multifamily Housing (Low-rise) - ITE No. 220				
	<b>Dwelling Units</b>	Trips Generated	Trips Entering	Trips Exiting
AM Peak	24	10	2	8
PM Peak	24	12	8	4

## **Appendix B**

# **SIGHT LINE CALCULATIONS**

**Site Accesses at Cabana Road West** 

#### 24-1642: 619 Cabana Rd. W., Windsor, Ontario - Sight Line Analysis

#### **Design Intersection Sight Distance (TAC Geometric Design Guide for Canadian Roads)**

Design Speed: 60km/h (Posted Speed Limit = 50 km/h)

Table 9.9.3: Time Gap for Case B1, Left Turn from Stop

Design Vehicle	Time Gap $(t_g)(s)$ at Design Speed of Major Road	
Passenger car	7.5	
Single-unit truck	9.5	
Combination truck (WB 19 and WB 20 )	11.5	
Longer truck	To be established by road authority	

Intersection Stopping Distance (ISD) = 0.278 V<sub>major</sub> t<sub>g</sub>

#### Where:

ISD = intersection sight distance (m)

(length of the leg of sight triangle along the major road)

 $V_{major}$  = design speed of the major road (km/h)

 $t_{\mbox{\scriptsize g}}$  = time gap for minor road vehicle to enter the major road (s)

ISD passenger car (left turn from stop) =  $0.278 \times 60 \times 7.5 = 125 \text{ m}$ 

Table 9.9.5: Time Gap for Case B2—Right Turn from Stop and Case B3—Crossing Maneuver

Design Vehicle	Time Gap $(t_g)(s)$ at Design Speed of Major Road	
Passenger car	6.5	
Single-unit truck	8.5	
Combination truck (WB 19 and WB 20)	10.5	

ISD passenger car (right turn from stop) =  $0.278 \times 60 \times 6.5 = 108 \text{ m}$